The background is a vibrant red color. It features several abstract geometric shapes: a large white circle with a blue border in the upper right; a smaller white circle with a blue border in the lower left; a large green semi-circle in the bottom right; and various other shapes in blue, green, and white, some with thick borders, scattered throughout the page.

Appendix E
Geotechnical
Interpretation Report

**RINGSEND TO CITY CENTRE CORE BUS CORRIDOR
SCHEME
GEOTECHNICAL INTERPRETATIVE REPORT
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1. INTRODUCTION AND DESKTOP REVIEW

The existing site investigation information for the area has been taken from the Geological Survey of Ireland (GSI) website and the British Geological Survey (BGS) website, including the Quaternary and Bedrock Geology of Dublin and Depth of Bedrock digital maps.

The following selection of published papers has found to be of relevance to estimate the lithology and geotechnical properties:

- “Geotechnical properties of Dublin boulder clay”. Authors: Long, Michael M and Menkiti, Christopher O. Sept 2007, *Géotechnique* 57 (7): 595-611. Published by the ICE.
- Ground Investigation Report of the National Pediatric Hospital Project, Dublin. Roughan & O’Donovan Consulting Engineers, January 2015.

1.1 Overview of geotechnical conditions along the Project.

Quaternary sediments cover up to 80% of the Dublin region. Quaternary thicknesses at the city area range from 5 to 20m. Maximum thicknesses are recorded along a Tertiary channel occurring on the north shore of the River Liffey valley, reaching 45m, and along a channel-like feature running along the south margin of the Dodder valley Quaternary sediments, with a thickness of 15 to 25 m.

The most commonly occurring Quaternary deposit in the area has been termed locally as the Dublin Boulder Clay. It is a glacial deposit derived from the Lower Carboniferous Limestone and it is classified by its two main members: the Black Boulder Clay (BkBC) and the Brown Boulder Clay (BrBC). The Brown Boulder Clay is less consolidated and since it overlies the Black Boulder Clay it has been interpreted as its weathered upper layer.

The Upper Brown Boulder Clay (UBrBC) is the outcome of the oxidation of the clay particles in the top 2m to 3m of the UBkBC, resulting in a change in colour from black to brown and a lower strength material. It is usually described as thick stiff to very stiff brown, slightly sandy clay, with rare silt / gravel lenses and some rootlets, particularly in the upper metre.

The Upper Black Dublin Boulder Clay (UBkBC) is a very stiff, dark grey, slightly sandy clay, with some gravel and cobbles. It is typically 4 m to 12 m thick.

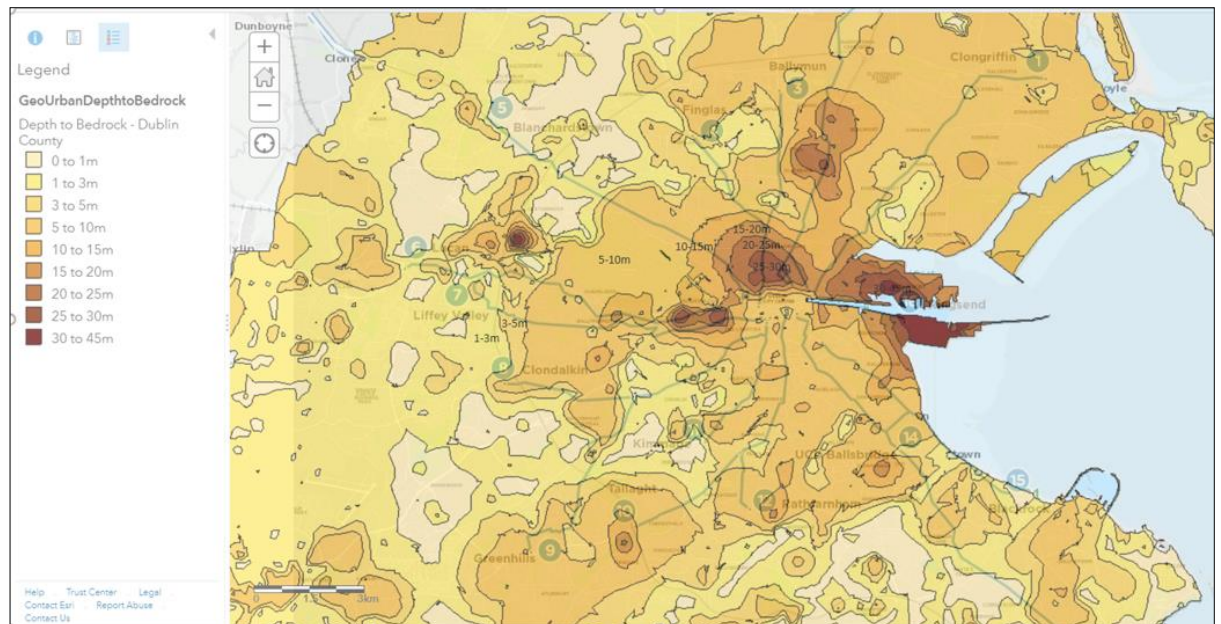
The Lower Brown Dublin Boulder Clay (LBrBC) exists as a 5 m to 9 m thick hard, brown, silty clay, with gravel, cobbles and boulders. It has previously been called the “sandy boulder clay” as it is similar to but siltier than the UBkBC above.

The Lower Black Dublin Boulder Clay (LBkBC) is a patchy layer of hard slightly sandy gravelly clay with an abundance of boulders. Its thickness does not exceed 4 m and is typically less than 2 m.

Note that not all four distinct formations of the Dublin Boulder Clay are always present. The upper two units though have been proven at all investigation sites across the city.

Bedrock close to the surface occurs mostly along the main riverbeds as well as the coastline and the higher ground areas of the Howth peninsula. The bedrock map of Ireland shows a wide variety of rock types which have originated at different periods of geological time. Underlying the project area consists of Lower Carboniferous Limestone of the Lucan Formation (Calp), which is typically described as a dark grey to black fine grained limestone.

The following image from the Geological Survey Ireland website shows the expected depth to Bedrock.



Depth of Bedrock from the Geological Survey Ireland website

The water pressures correspond to hydrostatic conditions with a groundwater table about 2m below ground level.

- *Summary of Desktop Review.*

The following preliminary lithology and geotechnical properties has been assumed based on the Desktop Review:

Layer	Depth	Thickness	Undrained shear strength, c_u (kPa)
Made ground / Urban / Alluvium	0 to 1 m	1	0
Upper Brown Boulder Clay, UBrBC	1 to 3 m	2	80
Upper Black Boulder Clay, UBkBC	3 to 10 m	7	200
Lower Brown Boulder Clay, LBrBC	10 to 18 m	8	400
Lower Black Boulder Clay, LBkBC	18 to 22 m	4	600
Bedrock	>22 m	N/A	>600

The expected depth to bedrock has been included in Section 2.

2. SUMMARY OF GROUND INVESTIGATION CONTRACT

At the date of this document, there are two GI contracts underway. Lot 1, which includes projects C and D, and Lot 2, which covers A and B projects.

Proposed ground investigation works aim to assess the geology of the site and determine the ground properties and conditions to enable the design of Bus Connects Core Bus Corridors. The GI provides for boreholes, trial pits, dynamic probes, standpipes/piezometer installation and monitoring, in-situ testing, geotechnical and environmental laboratory testing and preparation of a factual report, all in accordance with the “*Specification and Related Documents for Ground Investigation in Ireland*”.

At the Project D schemes (Ballymun/Finglas to City Centre, Kimmage to City Centre and Ringsend to City Centre), there are 21 proposed investigation points, consisting of Cable Percussion (CP) and Rotary Core (RC) boreholes as well as few windowless dynamic samples (WS) in restricted space areas. The location of these points can be found in the form of drawings in the “*BusConnects Detailed Ground Investigation – Stage 1 – LOT 1*”, February 2020.

In situ tests mainly include standard penetration tests. Laboratory tests mainly include particle size distribution, Atterberg limits, density and moisture content to identify soils and direct shear strength, triaxial CU or UU and uniaxial compression to determine the strength of the soil/rock.

For more details see the “*BusConnects Detailed Ground Investigation – Stage 1 – LOT 1*”, February 2020.

For the Ringsend to City Centre Core Bus Corridor Scheme, the following investigation points have been proposed:

Borehole Ref.	Expected Depth to Bedrock	Borehole Depth (m) – Cable Percussion	Borehole Depth (m) – Rotary Core
R16-CP01	10-15m	15	-
R16-CP02	10-15m	15	-
R16-CP03	15-20m	15	-
R16-CP04	15-20m	15	-

3. SUMMARY OF FACTUAL REPORT

The following factual report was issued as part of the Lot 1 GI:

- Detailed Stage 1 Lot 1 Route 16. June 2021

Completed investigation points are as summarised below:

Structure	Borehole Ref.	Expected Depth to Bedrock	Borehole Depth (m) – Cable Percussion	Borehole Depth (m) – Rotary Core	Notes
Ringsend 01	R16-CP01	10-15m	5.0	-	
	R16-CP02	10-15m	9.1	-	
Ringsend 03	R16-CP03	15-20m	12.3	-	
	R16-CP04	15-20m	13.5	-	

The GI works undertaken comprise 4 No. Cable Percussion Boreholes to a maximum depth of 13.5m BGL; 22 SPT tests at 1 metre intervals alternating with disturbed samples and 6 GWL recordings.

13 disturbed samples were taken at each change of soil consistency or between SPT tests and 4 undisturbed samples (UT100) where ground conditions permit. Geotechnical testing consisting of 13 moisture content, 2 Atterberg limits, 2 Bulk Density and 9 Particle Size Distribution. Soil strength testing consisted of 4 Vane tests and 4 Shear Box.

Environmental & Chemical testing consisted of 19 Suite E samples and 1 pH and organic matter content test.

4. OVERVIEW OF SOIL CLASSIFICATION

4.1 Made ground

Made Ground deposits were encountered beneath the Topsoil/Surfacing and were present to depths of between 2.50m and 5.30m BGL.

Made ground deposits were described generally as either brown, sandy gravelly Clay with cobbles or greyish brown clayey gravelly Sand with occasional cobbles and contained occasional fragments of concrete, plastic, red brick and wood.

Note that a culvert was encountered in borehole R16-CP02 between 3.0 and 5.3m, which was noted as a void on the log.

The Particle Size Distribution tests confirm that generally the Made ground deposits are well-graded graded with percentages of sands between 22% and 53% and percentages of gravels between 31% and 69%.

PH and total organic carbon (TOC) were determined at R16-CP04 at 0.5m depth. Organic matter content (OMC) was estimated from TOC. PH, TOC and OMC values were 9.3, 1.6% w/w C and 2.8% w/w respectively.

Asbestos was detected at 0.5m depth at borehole R16-CP03.

4.2 Cohesive deposits

Cohesive deposits were encountered beneath the Made Ground or interbedded with Granular Deposits and were described typically as grey slightly sandy silty CLAY.

The strength of the cohesive deposits was typically very soft till depths of 11.7mBGL.

Cohesive deposits found to be a CLAY of high plasticity, with a plasticity index ranging between 29% and 31%.

Particle Size Distribution tests confirm generally well-graded deposits with percentages of sands and gravels ranging between 11% and 15% and 2% and 5%, respectively.

4.3 Granular deposits

Granular deposits were encountered interbedded with cohesive deposits in the majority of holes and were typically described as either greyish sandy sub rounded to rounded fine to coarse GRAVEL with occasional cobbles or gravelly fine to coarse SAND.

Based on the SPT N values the deposits vary from loose to dense.

Particle Size Distribution tests confirm generally well-graded deposits with percentages of sands and gravels ranging between 18% and 58% and 33% and 69%, respectively.

5. SUMMARY OF GROUND INVESTIGATION INTERPRETATIVE REPORT

For Ringsend to City Centre CBC scheme, the following lithology and soil strength properties has been assumed based on the GI findings:

Layer	Depth (m)	SPT	Undrained shear strength, c_u (kPa)
Topsoil, Concrete	0 to 0.5	-	-
Made Ground: Brown Clay (possibly UBrBC) / Sand / Gravel	0.5 to 6	6	40
Very soft silty Grey Clay (only found in 2 out of 4 boreholes)	6 to 12	3.5	20
Gravel	Top level between 6 and 12m	50	325

- 2 Vane tests at Made Ground Sand layer, defined as brown very sandy Gravel or brown very gravelly Sand, have shown Peak shear strength values higher than 146 kPa.
- 2 Vane tests at soft silty clay layer, shown Peak shear strength values between 11 and 13 kPa.
- 2 Shear Box tests at Made Ground Sand layer, defined as brown silty (very) gravelly Sand, shown angle of peak shearing resistant values between 34 and 44 degrees and effective cohesion values between 4 and 13 kPa.

The geological geotechnical ground profile can be found at Appendix 1.

Ground parameters from in situ and lab tests are shown in Appendix 2.

6. HIDROGEOLOGY

Groundwater was noted during the investigation although the exploratory holes did not remain open for sufficiently long periods of time to establish the hydrogeological regime. However, standpipes were installed to allow the equilibrium groundwater level to be determined.

Groundwater levels recorded during the GI works are summarized below:

Date:	20/4/21	16/6/21
R16-CP01	4.46	4.72
R16-CP02	5.03*	-
R16-CP03	-	2.47
R16-CP04	3.73	4.40

* Water depth might be unrepresentative due to culvert

7. GEOTECHNICAL INPUT TO STRUCTURES

The following table shows the expected depth to bedrock, based on the data from the Desktop Review, as well as the depth of the encountered bedrock in the GI undertaken.

Note that most of the boreholes were terminated at a shorter length, before encountering the bedrock strata. Therefore, the expected depth to bedrock could not be confirmed.

Structure	Permanent loads / Variable loads (KN)	Borehole Ref.	Expected Depth to Bedrock	Depth to encountered Bedrock	Depth to N _{SPT} values of Refusal	Piles estimated length (m)
Ringsend 01 D=0.5m	294 / 623	R16-CP01	10-15m	-	5m	11.0
		R16-CP02	10-15m	-	6m	11.5
Ringsend 02 D=0.2m	50	R16-CP03	15-20m	-	12.5m	11.5
		R16-CP04	15-20m	-	12.5m	12.5
Ringsend 03 D=0.5m	210 / 604	R16-CP03	15-20m	-	12.5m	15.5
		R16-CP04	15-20m	-	12.5m	16.5

A preliminary number of the characteristic compressive resistance of piles has been obtained following the alternative procedure in accordance with the Eurocode 7 and the Irish National Annex. This procedure makes use of the ground parameters (such as the undrained shear strength, c_u) to estimate the shaft and base compressive resistance of piles.

c_u values have been derived from SPT values obtained in each borehole following the SPT- c_u relationship proposed by Stroud and Butler (1975). Calcs can be found at Appendix 3.

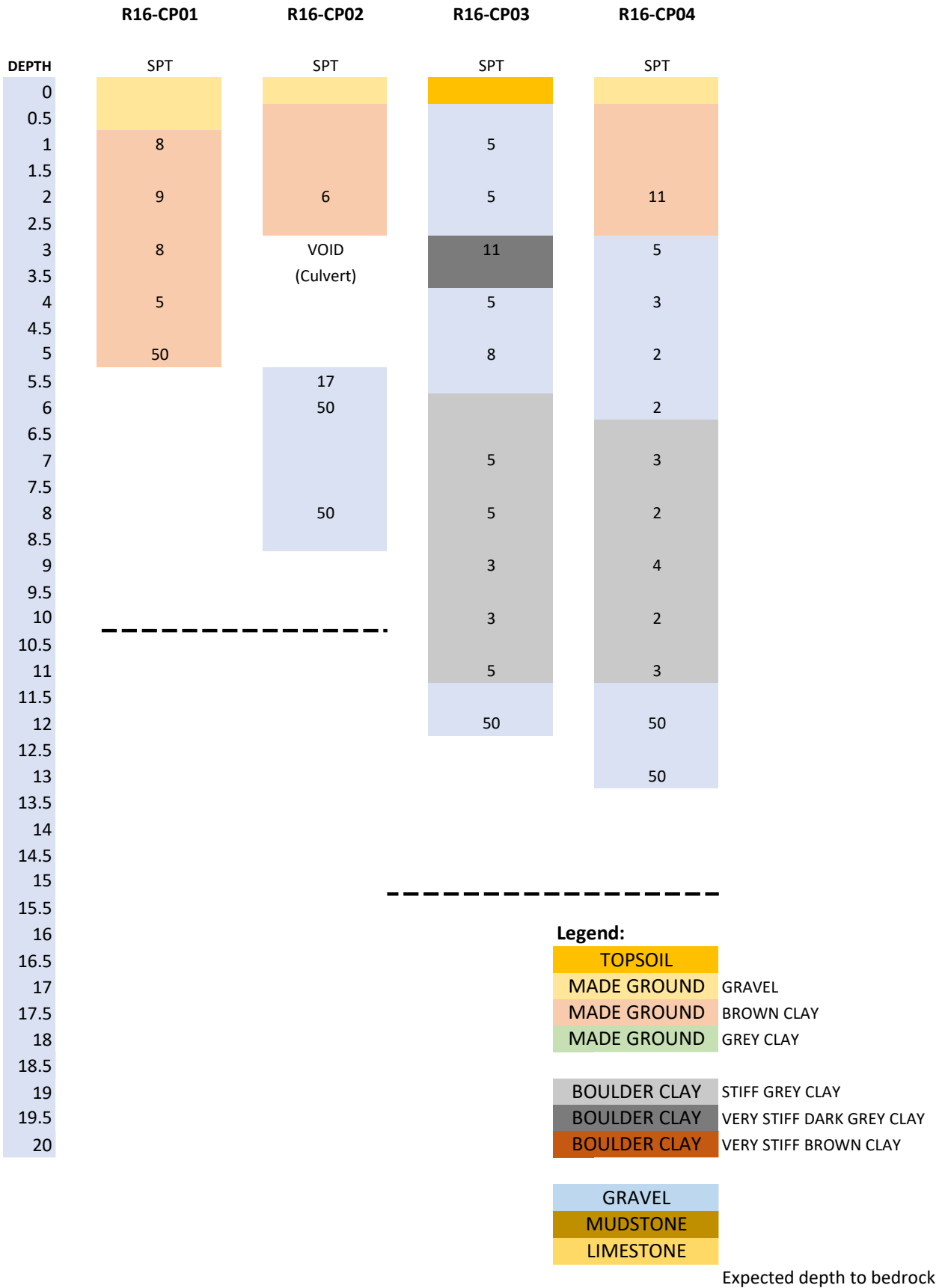
In Ringsend 01 and 03 0.5m diameter driven piles embedded in the Dublin boulder clay and Ringsend 02 0.2m piles, the estimated piles length that satisfies the ULS is as detailed in the table above.

8. APPENDICES

8.1 Geological geotechnical profile

RINGSEND 01

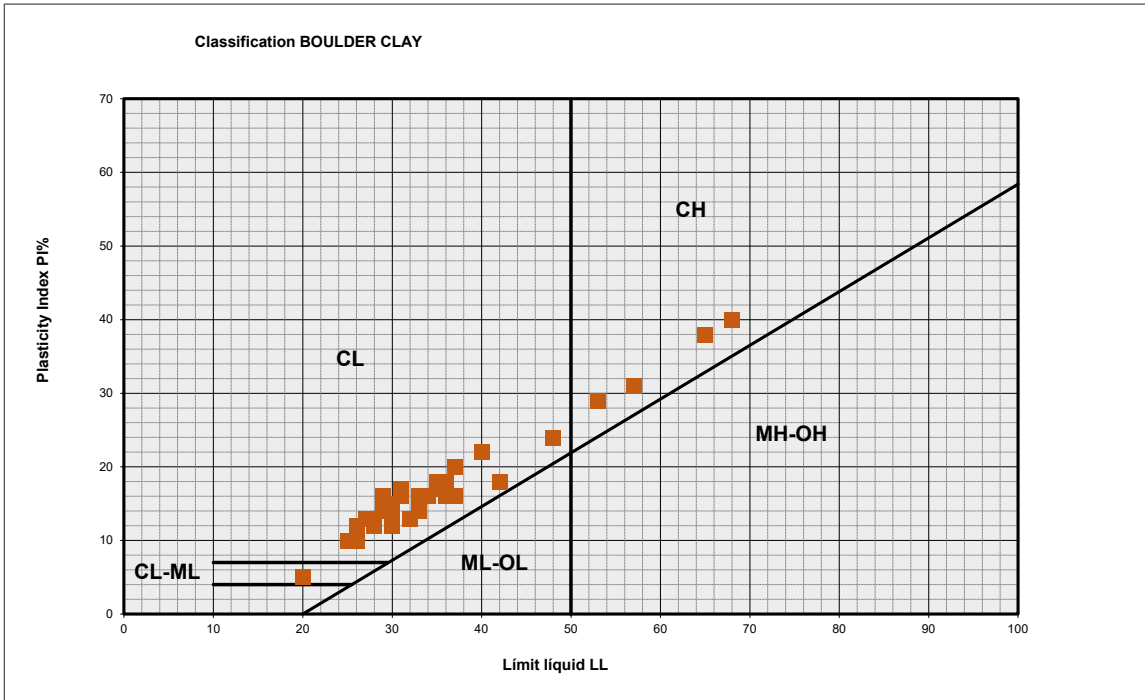
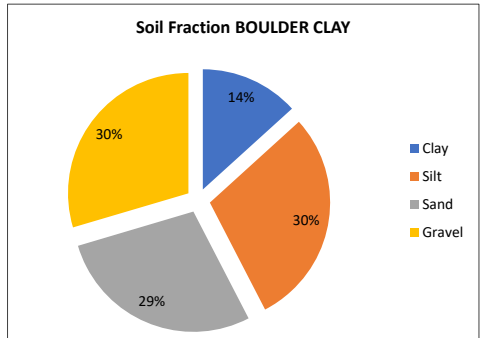
RINGSEND 02/03



8.2 Ground parameters

SOIL CLASSIFICATION

Borehole	Soil description	Sample type	Top depth (m)	Moisture Content %	Particle density Mg/m ³	Bulk density Mg/m ³	Liquid limit %	Plastic limit %	Plasticity Index %	Passing 0.425mm %	Classification	Particle size distribution - Soil Fraction						
												Clay	Silt	Sand	Gravel	Cobbles	Total	
Glasnevin BH01	-	CB	2.1	9.5	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	Dark grey slightly sandy gravelly silty CLAY	CB	3.6	10.1	-	-	34	18	16	33.1	CL	-	8%	15%	18%	58%	0%	100%
Glasnevin BH01	-	CB	5.1	7.6	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	Dark grey slightly sandy slightly gravelly silty CLAY	CB	6.6	7.8	-	-	-	-	-	-	-	-	10%	25%	32%	33%	0%	100%
Glasnevin BH01	-	CB	8.1	6.7	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	Dark grey silty very sandy fine to coarse GRAVEL	C	8.8	6.2	-	-	20	15	5	34	CL-ML	-	4%	17%	35%	44%	0%	100%
Glasnevin BH01	-	CB	9.6	29.7	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	-	CB	11.1	16.1	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	Dark grey slightly sandy gravelly silty CLAY	C	11.9	11	-	-	36	18	18	55.8	CL	-	17%	29%	15%	39%	0%	100%
Glasnevin BH01	-	CB	12.6	8.8	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	-	CB	14.1	9.6	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	-	CB	15.6	9.1	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH01	Dark grey/brown slightly gravelly slightly sandy silty CLAY	C	16.4	9.6	-	-	33	17	16	72.3	CL	-	17%	30%	35%	18%	0%	100%
Glasnevin BH02A	-	CB	2.4	12.3	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH02A	Dark grey slightly gravelly slightly sandy silty CLAY.	C	4.7	8.1	-	-	30	18	12	60.6	CL	-	14%	31%	31%	24%	0%	100%
Glasnevin BH02A	-	CB	6.9	8	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH02A	Dark grey slightly sandy slightly gravelly silty CLAY	C	10.95	9.3	-	-	36	19	17	57.8	CL	-	14%	30%	25%	31%	0%	100%
Glasnevin BH02A	Dark grey slightly sandy slightly gravelly silty CLAY	C	16.5	9.3	-	-	35	17	18	55.8	CL	-	14%	30%	22%	34%	0%	100%
Glasnevin BH02A	-	CB	17.4	9.6	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH02A	-	CB	20.4	10	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH02A	Dark grey slightly sandy slightly gravelly silty CLAY	C	22	9.8	-	-	37	17	20	67.1	CL	-	15%	35%	25%	25%	0%	100%
Glasnevin BH02A	-	CB	23.4	9.9	-	-	-	-	-	-	-	-	-	-	-	-	0%	
Glasnevin BH02A	Dark grey slightly gravelly slightly sandy silty CLAY	C	25.2	12.3	-	-	40	18	22	78.9	CL	-	19%	41%	25%	14%	0%	100%
Metrolink GBH01	Greyish brown sandy gravelly silty CLAY	B	1.2	12	-	-	33	19	14	69	CL	Low Plasticity CL	15%	33%	30%	22%	0%	100%
Metrolink GBH01	Greyish brown sandy gravelly silty CLAY	B	2	12	-	-	32	19	13	63	CL	Low Plasticity CL	17%	31%	30%	23%	0%	101%
Metrolink GBH01	Greyish brown sandy gravelly CLAY	B	4	12	-	-	26	15	11	64	CL	Low Plasticity CL	17%	35%	32%	17%	0%	101%
Metrolink GBH01	Greyish brown sandy gravelly CLAY	B	6.6	7.7	-	-	26	15	11	62	CL	Low Plasticity CL	10%	33%	39%	19%	0%	101%
Metrolink GBH01	Greyish brown sandy gravelly CLAY	B	9.4	19	-	-	27	14	13	44	CL	Low Plasticity CL	8%	20%	51%	21%	0%	100%
Metrolink GBH01	-	C	12	8.1	-	-	26	16	10	57	CL	Low Plasticity CL	17%	34%	36%	13%	0%	100%
Metrolink GBH01	-	B	16.6	7	-	-	-	-	-	-	-	-	-	4%	23%	54%	20%	101%
Metrolink GBH01	-	B	17.2	8.2	-	-	25	15	10	58	CL	Low Plasticity CL	17%	35%	36%	13%	0%	101%
Metrolink GBH02	Greyish brown sandy gravelly silty CLAY	B	2	16	-	-	48	24	24	70	CI	Low Plasticity CL	14%	33%	30%	23%	0%	100%
Metrolink GBH02	Greyish brown sandy gravelly silty CLAY	B	4	11	-	-	29	14	15	60	CL	Low Plasticity CL	16%	34%	30%	20%	0%	100%
Metrolink GBH02	Greyish brown sandy gravelly CLAY	B	6	8.1	-	-	25	15	10	58	CL	Low Plasticity CL	13%	34%	32%	21%	0%	100%
Metrolink GBH02	Grey very gravelly very sandy CLAY	C	9.6	10	-	-	30	16	14	51	CL	Low Plasticity CL	15%	21%	29%	35%	0%	100%
Metrolink GBH02	Greyish brown sandy gravelly CLAY	B	14	10	-	-	30	15	15	72	CL	Low Plasticity CL	-	-	-	-	0%	
Metrolink GBH02	Greyish brown sandy gravelly CLAY	B	16	10	-	-	28	15	13	63	CL	Low Plasticity CL	12%	25%	45%	19%	0%	101%
Metrolink GBH21	Greyish brown sandy gravelly silty CLAY	B	1.2	8.5	-	-	26	15	11	50	CL	Low Plasticity CL	10%	40%	38%	13%	0%	101%
Metrolink GBH22	Greyish brown sandy gravelly silty CLAY	B	1.2	8.7	-	-	26	14	12	46	CL	Low Plasticity CL	13%	34%	35%	19%	0%	101%
Metrolink GBH28	Greyish brown sandy gravelly silty CLAY	B	1.2	8	-	-	28	16	12	55	CL	Low Plasticity CL	10%	33%	40%	18%	0%	101%
R03-CP03	Dark brown mottled grey sandy slightly gravelly CLAY	B	2	17	-	-	-	-	-	-	-	-	-	34%	42%	24%	0%	100%
R03-CP03	Brown slightly sandy slightly gravelly CLAY	B	3	19	-	-	-	-	-	-	-	-	-	46%	25%	29%	0%	100%
R03-CP03	Dark brown mottled grey slightly sandy slightly gravelly CLAY	B	5	12	2.64	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP03	Dark brown mottled grey slightly sandy slightly gravelly CLAY	B	6	12	-	-	29	15	14	64	CL	Low Plasticity CL	-	49%	31%	20%	0%	100%
R03-CP07	Brown slightly sandy gravelly CLAY	B	1.5	23	-	-	35	18	17	43	CI	Intermediate Plasticity CI	-	29%	24%	47%	0%	100%
R03-CP07	Brown slightly sandy gravelly CLAY	B	3	16	2.6	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP07	Brown mottled grey slightly sandy slightly gravelly CLAY with many cobbles.	B	5	12	-	-	29	15	14	31	CL	Low Plasticity CL	-	24%	14%	27%	35%	100%
R03-CP07	Brown slightly sandy gravelly CLAY	B	6	14	2.7	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP08	Brown slightly sandy slightly gravelly CLAY	B	2.2	31	2.64	-	68	28	40	65	CH	High Plasticity CH	27%	33%	10%	30%	0%	100%
R03-CP08	Stiff brown slightly sandy gravelly CLAY	U	3.5	9.4	2.62	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP08	Brown slightly sandy very gravelly CLAY	B	4	11	-	-	31	15	16	50	CL	Low Plasticity CL	17%	23%	23%	37%	0%	100%
R03-CP14	MADE GROUND dark grey very gravelly clayey sand of ash.	B	2	38	2.03	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP14	Dark grey gravelly slightly sandy CLAY with some organic material	B	3	49	-	-	65	27	38	51	CH	High Plasticity CH	15%	27%	23%	35%	0%	100%
R03-CP14	Dark grey very gravelly very sandy CLAY.	B	4	15	2.59	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP14	Dark grey sandy very clayey GRAVEL	B	5	10	-	-	29	13	16	36	CL	Low Plasticity CL	11%	17%	16%	56%	0%	100%
R03-CP14	Dark grey very gravelly very sandy CLAY.	B	6	11	2.61	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP14	Dark grey gravelly slightly sandy CLAY	B	7	14	-	-	31	14	17	51	CL	Low Plasticity CL	16%	25%	22%	37%	0%	100%
R03-CP14	Dark grey very gravelly very sandy CLAY.	B	8	13	2.6	-	-	-	-	-	-	-	-	-	-	-	0%	
R03-CP14	Brown slightly sandy gravelly CLAY	B	9	13	-	-	-	-	-	-	-	-	-	-	-	-	0%	
R11-CP01	Grey brown slightly sandy slightly gravelly silty CLAY	B	2	15.6	-	-	42	24	18	59	CL	-	13%	32%	24%	31%	0%	100%
R11-CP01	Grey brown slightly sandy slightly gravelly silty CLAY	B	2.5	14.2	-	-	-	-	-	-	-	-	-	45%	24%	32%	0%	100%
R11-CP01	Grey brown slightly sandy slightly gravelly silty CLAY	B	4	15.9	-	-	37	21	16	59	CL	-	0%	31%	28%	28%	0%	87%
R11-CP01	Dark grey slightly gravelly slightly sandy silty CLAY	B	5.5	13.5	-	-	-	-	-	-	-	-	-	45%	28%	27%	0%	100%
R11-CP01	Dark grey slightly sandy slightly gravelly silty CLAY	B	6	13.6	-	-	30	18	12	54.5	CL	-	13%	27%	26%	34%	0%	100%
R11-CP01	Dark grey slightly sandy slightly gravelly silty CLAY	B	7.5	13.3	-	-	34	18	16	58.2	CL	-	14%	30%	28%	28%	0%	100%
R11-CP01	Dark grey slightly sandy slightly gravelly silty CLAY	B	8	14.3	-	-	-	-	-	-	-	-	-	41%	25%	34%	0%	100%
R11-CP03	MADE GROUND brownish grey very gravelly very sandy CLAY	B	2	23	-	-	36	20	16	49	CI	Intermediate Plasticity CI	-	-	-	-	0%	
R11-CP03	Grey very sandy clayey GRAVEL	B	3	15	2.67	-	-	-	-	-	-	-	-	-	-	-	0%	
R11-CP03	-	B	4.48	0.3	-	2.71	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP01	Brown slightly clayey silty very sandy GRAVEL	B	1.2	11	2.61	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP01	Brown silty very sandy GRAVEL	B	2	4.7	-	-	-	NP	-	-	-	-	-	17%	30%	40%	13%	100%
R16-CP02	Brown very gravelly SAND	B	1	13	2.64	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP02	Brown silty very sandy GRAVEL	B	2	9.7	-	-	-	NP	-	-	-	-	-	6%	25%	69%	0%	100%
R16-CP02	Brown & grey silty SAND	U100	6.5	19	-	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP02	Brown mottled grey slightly sandy slightly gravelly CLAY with some organic material	U100	7.5	41	-	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP02	Brown slightly silty very sandy GRAVEL	B	8	3.5	2.62	-	-	NP	-	-	-	-	-	2%	29%	69%	0%	100%
R16-CP04	Brown silty very sandy GRAVEL with many cobbles	B	1	-	2.68	-	-	-	-	-	-	-	-	9%	22%	35%	34%	100%
R16-CP04	Brown slightly clayey silty very gravelly SAND	B	3	-	2.62	-	-	-	-	-	-	-	-	16%	53%	31%	0%	100%
R16-CP04	Brown slightly clayey silty very gravelly SAND	B	4	14	2.65	2.06	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP04	Brown silty very gravelly SAND	B	5	-	2.66	-	-	-	-	-	-	-	-	9%	58%	33%	0%	100%
R16-CP04	Brown silty gravelly SAND	B	6	6.4	2.65	1.7	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP04	Brown & grey silty SAND	U100	6.5	19	2.58	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP04	Brown mottled grey slightly sandy slightly gravelly CLAY with some organic material	B	7	47	-	-	57	26	31	90	CH	High plasticity CH	-	84%	11%	5%	0%	100%
R16-CP04	Brown mottled grey slightly sandy slightly gravelly CLAY with some organic material	U100	7.5	41	2.52	-	-	-	-	-	-	-	-	-	-	-	0%	
R16-CP04	Brown mottled grey slightly sandy slightly gravelly CLAY with some organic material	B	8	46	-	-	53	24	29	93	CH	High plasticity CH	-	83%	15%	2%	0%	100%
R16-CP04	Brownish grey silty sandy GRAVEL with cobbles	B	12	-	2.69	-	-	-	-	-	-	-	-	5%	18%	67%	10%	100%



SOIL STRENGTH

Test	BH	Top depth (m)	Soil	N _{pr}	Correlation factor	Cu (KPa)
R11-CP04		1	MADE GROUND: Brown slightly sandy slightly gravelly silty CLAY. Gravel is angular to sub rounded fine to coarse with occasional fragments of brick and concrete.	10		65
R11-CP04		2	Medium dense greyish brown sandy sub angular to rounded fine to coarse GRAVEL	27		175.5
R11-CP04		2.6	Firm brownish grey slightly sandy gravelly CLAY. Gravel is angular to sub rounded fine to coarse	50		325
R11-CP01		1.2	MADE GROUND: Greyish brown slightly sandy gravelly Clay with occasional sub-angular to sub-rounded cobbles, red brick and mortar fragments.	2		13
R11-CP01		2	MADE GROUND: Greyish brown slightly sandy gravelly Clay with occasional sub-angular to sub-rounded cobbles, red brick and mortar fragments.	4		26
R11-CP01		3	MADE GROUND: Greyish brown slightly sandy gravelly Clay with occasional sub-angular to sub-rounded cobbles, red brick and mortar fragments.	13		84.5
R11-CP01		4	Very stiff brown slightly sandy slightly gravelly CLAY with occasional sand lenses.	35		227.5
R11-CP01		5	Very stiff dark grey slightly sandy gravelly CLAY with rare sub-rounded cobbles.	28		182
R11-CP01		6	Very stiff dark grey slightly sandy gravelly CLAY with rare sub-rounded cobbles.	31		201.5
R11-CP01		7	Very stiff dark grey slightly sandy gravelly CLAY with rare sub-rounded cobbles.	35		227.5
R11-CP01		8	Very stiff dark grey slightly sandy gravelly CLAY with rare sub-rounded cobbles.	50		325
R11-CP03		1	MADE GROUND: Dark brown slightly sandy gravelly Clay with occasional angular to subrounded cobbles and occasional fragments of glass, metal, red brick and wood (creosote like odour)	3		19.5
R11-CP03		2	MADE GROUND: Grey slightly sandy slightly gravelly Clay with occasional angular to subangular cobbles and occasional fragments of red brick and wood	9		58.5
R11-CP03		3	MADE GROUND: Grey slightly sandy slightly gravelly Clay with occasional angular to subangular cobbles and occasional fragments of red brick and wood	50		325
R16-CP01		1.2	MADE GROUND: Brown slightly sandy gravelly Clay with some subangular to rounded cobbles, occasional boulders and occasional fragments of red brick	8		52
R16-CP01		2	MADE GROUND: Brown slightly sandy gravelly Clay with some subangular to rounded cobbles, occasional boulders and occasional fragments of red brick	9		58.5
R16-CP01		3	MADE GROUND: Brown slightly sandy gravelly Clay with some subangular to rounded cobbles, occasional boulders and occasional fragments of red brick	8		52
R16-CP01		4	MADE GROUND: Brown slightly sandy gravelly Clay with some subangular to rounded cobbles, occasional boulders and occasional fragments of red brick	5		32.5
R16-CP01		5	MADE GROUND: Brown slightly sandy gravelly Clay with some subangular to rounded cobbles, occasional boulders and occasional fragments of red brick	50		325
R16-CP02		2	MADE GROUND: Brown slightly sandy gravelly Clay with occasional angular to subrounded cobbles and occasional fragments of concrete and red brick	6		39
R16-CP02		5.3	MADE GROUND: Brown sandy clayey angular to rounded fine to coarse GRAVEL with occasional fragments of red brick (driller's notes)	17		110.5
R16-CP02		6	Dense grey slightly clayey very sandy subangular to rounded fine to coarse GRAVEL with some subangular to rounded cobbles	50		325
R16-CP02		8	Dense grey slightly clayey very sandy subangular to rounded fine to coarse GRAVEL with some subangular to rounded cobbles	50		325
R16-CP03		1	MADE GROUND: Brown silty gravelly Sand with occasional cobbles and boulders, and occasional fragments of concrete, plastic, wood and red brick	6		39
R16-CP03		2	POSSIBLE MADE GROUND: Brown slightly gravelly clayey SAND.	5		32.5
R16-CP03		3	Soft dark grey slightly sandy very gravelly CLAY with occasional sub angular to sub rounded cobbles. Gravel is subrounded to rounded fine to coarse	11		71.5
R16-CP03		4	Very loose grey very sandy subangular to rounded fine to coarse GRAVEL	5		32.5
R16-CP03		5	Very loose grey very sandy subangular to rounded fine to coarse GRAVEL	8		52
R16-CP03		7	Very soft grey slightly sandy silty CLAY with occasional shell fragments and organic matter.	5		32.5
R16-CP03		8	Very soft grey slightly sandy silty CLAY with occasional shell fragments and organic matter.	5		32.5
R16-CP03		9	Very soft grey slightly sandy silty CLAY with occasional shell fragments and organic matter.	3		19.5
R16-CP03		10	Very soft grey slightly sandy silty CLAY with occasional shell fragments and organic matter.	3		19.5
R16-CP03		11	Very soft grey slightly sandy silty CLAY with occasional shell fragments and organic matter.	50		325
R16-CP03		12	Dense grey sandy subrounded to rounded fine to coarse GRAVEL with some subangular to rounded cobbles.	50		325
R16-CP04		2	MADE GROUND: Brown slightly gravelly sandy CLAY with some cobbles and occasional fragments of red brick	11		71.5
R16-CP04		3	MADE GROUND: Greyish brown clayey gravelly fine to coarse SAND. Gravel is subangular to rounded fine to coarse	5		32.5
R16-CP04		4	MADE GROUND: Greyish brown clayey gravelly fine to coarse SAND. Gravel is subangular to rounded fine to coarse	3		19.5
R16-CP04		5	Very loose dark grey clayey gravelly fine to coarse SAND. Gravel is subrounded to rounded fine to coarse	2		13
R16-CP04		6	Very loose grey very gravelly fine to coarse SAND. Gravel is subrounded to rounded fine to coarse	2		13
R16-CP04		7	Very soft grey slightly sandy silty CLAY with occasional shell fragments	3		19.5
R16-CP04		8	Very soft grey slightly sandy silty CLAY with occasional shell fragments	2		13
R16-CP04		9	Very soft grey slightly sandy silty CLAY with occasional shell fragments	4		26
R16-CP04		10	Very soft grey slightly sandy silty CLAY with occasional shell fragments	2		13
R16-CP04		11	Very soft grey slightly sandy silty CLAY with occasional shell fragments	3		19.5
R16-CP04		12	Dense grey slightly clayey very sandy subrounded to rounded fine to coarse GRAVEL with some subangular to rounded cobbles	50		325
R16-CP04		13	Dense grey slightly clayey very sandy subrounded to rounded fine to coarse GRAVEL with some subangular to rounded cobbles	50		325
Glasnevin BH01		2.1	Stiff grey sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is fine to coarse subangular to subrounded	19		123.5
Glasnevin BH01		3.6	Very stiff grey sandy gravelly CLAY with occasional subangular to subrounded cobbles and boulders. Gravel is fine to coarse subangular to subrounded	41		266.5
Glasnevin BH01		5.1	Driller notes gravelly CLAY - Recovery consists subangular to subrounded cobbles and boulders of Limestone	50		325
Glasnevin BH01		6.6	Very stiff grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded.	50		325
Glasnevin BH01		8.1	Very stiff grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded.	50		325
Glasnevin BH01		9.6	Very stiff grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded.	50		325
Glasnevin BH01		11.1	Very stiff grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded.	50		325
Glasnevin BH01		14.1	Very stiff brown grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded	50		325
Glasnevin BH01		15.6	Very stiff brown grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded	50		325
Glasnevin BH01		17.1	Very stiff brown grey slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel fine to coarse subangular to subrounded	50		325
Glasnevin BH02A		2.4	Gravelly band. Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	44		286
Glasnevin BH02A		3.9	Gravelly band. Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		5.4	Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		6.9	Gravelly band. Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		8.4	Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		9.9	Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		11.25	Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		15.9	Very stiff dark grey slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse.	50		325
Glasnevin BH02A		17.4	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		18.7	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		2.4	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		21.9	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		23.4	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		24.9	Very stiff brown slightly sandy gravelly CLAY. Gravel subangular to subrounded fine to coarse	50		325
Glasnevin BH02A		26.4	Dense grey fine to coarse angular to subangular GRAVEL with some angular to subrounded cobbles. Fines may have washed out	50		325
Metrolink GBH01		1.2	Sand is fine to coarse. Gravel is subangular fine to coarse of mixed lithologies. Cobbles and boulders are subrounded to subangular of mixed lithologies.	8	6.5	52
Metrolink GBH01		3	Stiff becoming very stiff black slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium of mixed lithologies.	29		188.5
Metrolink GBH01		5	Very stiff brownish grey sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of limestone.	50		325
Metrolink GBH01		8	Very stiff brownish grey sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of limestone.	50		325
Metrolink GBH01		11	Greyish brown slightly silty gravelly fine to coarse SAND. Gravel is subangular fine to coarse of limestone and sandstone.	50		325
Metrolink GBH01		14	Grey and brown slightly sandy subangular ne to coarse GRAVEL of limestone and sandstone with low cobble content. Sand is ne to coarse. Cobbles are subangular of limestone.	50		325
Metrolink GBH02		1.2	MADE GROUND: Stiff greyish brown slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium of mixed lithologies.	20		130
Metrolink GBH02		3	Very stiff greyish black slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subangular to subrounded fine to medium of mixed lithologies.	32		208
Metrolink GBH02		5	Firm becoming stiff brownish grey slightly sandy slightly gravelly silty CLAY. Sand is ne to coarse. Gravel is subangular ne to medium of predominantly limestone.	50		325
Metrolink GBH21		1.2	Very stiff greyish black slightly sandy silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of mixed lithologies.	50		325
Metrolink GBH21		1.7	Very stiff greyish black slightly sandy silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of mixed lithologies.	50		325
Metrolink GBH22		1.2	Very stiff greyish black slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subrounded fine to medium of mixed lithologies.	50		325
Metrolink GBH22		1.8	Very stiff greyish black slightly sandy slightly gravelly silty CLAY. Sand is fine to coarse. Gravel is subrounded fine to medium of mixed lithologies.	50		325
Metrolink GBH28		1.2	Very stiff greyish black slightly sandy silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of mixed lithologies.	45		292.5
Metrolink GBH28		2	Very stiff greyish black slightly sandy silty CLAY. Sand is fine to coarse. Gravel is subangular fine to medium of mixed lithologies.	50		325
R3-CP08		1.2	MADE GROUND: Brownish grey sandy clayey angular to subrounded fine to coarse Gravel with occasional fragments of ceramic, rubber and red brick	4		26
R3-CP08		2	MADE GROUND: Brownish grey sandy clayey angular to subrounded fine to coarse Gravel with occasional fragments of ceramic, rubber and red brick	5		32.5
R3-CP08		3	Firm brownish grey slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to coarse	11		71.5
R3-CP08		4	Stiff greyish brown slightly sandy gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	27		175.5
R03-CP03		1	MADE GROUND: Grey slightly sandy gravelly Clay with occasional angular to subangular cobbles and occasional fragments of red brick	6		39
R03-CP03		2	MADE GROUND: Grey slightly sandy gravelly Clay with occasional angular to subangular cobbles and occasional fragments of red brick	4		26
R03-CP03		3	Firm brown slightly sandy slightly gravelly CLAY. Gravel is subangular to subrounded fine to coarse	12		78
R03-CP03		4	Very stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	48		312
R03-CP03		5	Very stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP07		2	MADE GROUND: Greyish brown slightly sandy slightly gravelly Clay with occasional rootlets and occasional fragments of red brick	2		13
R03-CP07		3	Stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	18		117
R03-CP07		4	Very stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	38		247
R03-CP07		5	Very stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP07		6	Very stiff grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP14		1	MADE GROUND: Dark grey slightly sandy gravelly Clay with occasional angular to subangular cobbles and occasional fragments of brick, metal and wood	1		6.5
R03-CP14		2	MADE GROUND: Dark grey slightly sandy gravelly Clay with occasional angular to subangular cobbles and occasional fragments of brick, metal and wood	1		6.5
R03-CP14		3	MADE GROUND: Dark grey slightly sandy gravelly Clay with occasional angular to subangular cobbles and occasional fragments of brick, metal and wood	21		136.5
R03-CP14		4	Stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	23		149.5
R03-CP14		5	Very stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP14		6	Very stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP14		7	Very stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP14		8	Very stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-CP14		9	Very stiff dark grey slightly sandy slightly gravelly CLAY with occasional subangular to subrounded cobbles. Gravel is subangular to subrounded fine to coarse	50		325
R03-RC01		2	MADE GROUND: Dark grey slightly sandy gravelly Clay with red brick fragments.	3		19.5
R03-RC01		3.5	MADE GROUND: Dark grey slightly sandy gravelly Clay with red brick fragments.	2		13
R03-RC01		5	Stiff dark grey slightly sandy gravelly CLAY with occasional cobbles. (Drillers notes: Boulder Clay)	18		117
R03-RC01		6.5	Stiff dark grey slightly sandy gravelly CLAY with occasional cobbles. (Drillers notes: Boulder Clay)	21		136.5
R03-RC01		8	Stiff dark grey slightly sandy gravelly CLAY with occasional cobbles. (Drillers notes: Boulder Clay)	22		143
R03-RC01		9.5	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC01		11	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC01		12.5	Boulder Clay. Very stiff brown slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC01		14	Very stiff brown slightly sandy slightly gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-RC01		15.5	Very stiff brown slightly sandy slightly gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-RC01		17	Very stiff brown slightly sandy slightly gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-RC01		18.5	Very stiff dark brownish grey slightly sandy slightly gravelly CLAY.	50		325
R03-RC02		2	MADE GROUND: Dark grey sandy gravelly Clay with occasional cobbles.	5		32.5
R03-RC02		3.5	MADE GROUND: Dark grey sandy gravelly Clay with occasional cobbles.	22		143
R03-RC02		5	MADE GROUND: Dark grey sandy gravelly Clay with occasional cobbles.	28		182
R03-RC02		6.5	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		8	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		9.5	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		11	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		12.5	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		14	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		15.5	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		17	Boulder Clay. Very stiff dark grey slightly sandy gravelly CLAY with occasional cobbles.	50		325
R03-RC02		18.5	MUDSTONE	50		325
R03-CP03		2	MADE GROUND: Concrete fragments, red brick fragments and angular fine to coarse Gravel	17		110.5
R03-CP03		3.5	MADE GROUND: Concrete fragments, red brick fragments and angular fine to coarse Gravel	19		123.5
R03-CP03		5	Stiff dark grey slightly sandy gravelly CLAY with sub angular to sub rounded occasional cobbles.	20		130
R03-CP03		6.5	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	35		227.5
R03-CP03		8	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		9.5	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		11	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		12.5	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		14	Very stiff dark grey slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		15.5	Very stiff brown slightly sandy gravelly CLAY with occasional sub angular to sub rounded cobbles.	50		325
R03-CP03		17	Grey slightly clayey sandy fine to coarse sub-angular to sub-rounded GRAVEL.	50		325

NOTE: Cu=f1-N_{pr}. Correlation factor f1 according to Stroud and Butler (1975), taking a plastic

SOIL STRENGTH

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Peak shear strength KPa
Vane Test	R16-CP01	Brown slightly clayey very sandy GRAVEL	B	3.00	-	>146
	R16-CP02	Dark brown very gravelly SAND	B	3.00	-	>146
	R16-CP02	Brown & grey silty SAND	U100	6.50	19.00	13.00
	R16-CP02	Brown mottled grey slightly sandy slightly gravelly CLAY with some organ	U100	7.50	41.00	11.00
	R03-CP03	Dark brown mottled grey sandy slightly gravelly CLAY	B	2.00	17.00	19.00
	R03-CP03	Brown slightly sandy slightly gravelly CLAY	B	3.00	19.00	20.00

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Bulk density Mg/m ³	Cu KPa	Load failure (deviator) kPa	Failure Mode	Strain %	Effective angle of friction (°)	Effective cohesion kPa
Triaxial CU	Glasnevin BH01	Stiff dark grey silty very sandy fine to coarse GRAVEL	C	8.80	7.70	2.40	180.30	360.60	brittle	4.29	-	-
	Glasnevin BH01	Dark grey slightly sandy gravelly silty CLAY.	C	11.90	24.80	2.02	32.40	64.90	plastic	13.24	-	-
	Glasnevin BH01	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY	C	14.65	8.50	2.35	682.00	1364.00	plastic	8.85	-	-
	Glasnevin BH02A	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY	C	4.70	8.10	2.39	516.60	1033.30	brittle	4.98	-	-
	Glasnevin BH02A	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY	C	10.95	9.20	2.32	475.90	951.80	brittle	4.63	-	-
	Glasnevin BH02A	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY	C	16.50	8.30	2.40	662.80	1325.50	brittle	5.61	-	-
	Glasnevin BH02A	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY.	C	22.00	9.80	2.32	670.80	1341.70	brittle	3.66	-	-
	Glasnevin BH02A	Extremely stiff dark grey slightly sandy slightly gravelly silty CLAY	C	25.20	8.70	2.25	503.00	1006.00	brittle	7.56	-	-
	Metrolink GBH01	Greyish brown sandy gravelly silty CLAY	-	12.00	8.50	2.37	827.00	1654.00	brittle	12.50	-	-
Triaxial CU with PWP	Glasnevin BH02A	Stiff dark grey slightly sandy slightly gravelly silty CLAY	-	6.00	8.20	2.37	-	-	-	-	0.00	38.10
	Glasnevin BH02A	Stiff dark brown slightly sandy slightly gravelly silty CLAY	-	18.50	8.70	2.38	-	-	-	-	0.00	37.00
	Metrolink GBH02	Stiff brownish grey sandy gravelly silty CLAY	-	9.60	10.00	2.26	-	-	-	-	27.70	86.49
Triaxial UU	R03-CP08	Stiff brown slightly sandy gravelly CLAY	U	3.50	9.40	2.20	82.00	163.00	plastic	18.50	-	-

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Bulk density Mg/m ³	Peak Shear Stress KPa	Displacement at peak shear stress mm	Angle of shearing resistance	Effective cohesion KPa
Shear Box	R16-CP02	Gravel	B	7.00	-	-	-	-	-	-
	R16-CP02	Gravel	B	9.00	-	-	-	-	-	-
	R16-CP04	Brown slightly clayey silty very gravelly SAND	B	4.00	14.00	2.06	50-101-196	3-3-4	44.00	4.00
	R16-CP04	Brown silty gravelly SAND	B	6.00	6.40	1.70	46-49-147	4-4-6	34.00	13.00
	Metrolink BH01	Brown gravelly sandy CLAY	B	2.00	13.00	1.93	16-29-50	9.31-7.81-8.71	29.00	6.00
	Metrolink BH01	Gravel	B	17.20	20.00	1.96	124-231-459	2.4-9.6-6.61	34.00	0.00
	R03-CP03	Dark brown mottled grey slightly sandy slightly gravelly CLAY	B	5.00	12.00	2.24	45-79-138	5-5-4	32.00	15.00
	R03-CP14	Brown slightly sandy gravelly CLAY	B	9.00	13.00	2.32	39-80-150	4.51-4.8-8.1	36.00	5.00

ROCK CLASSIFICATION

Borehole	Top depth (m)	Soil	TCR	SCR	RQD	FI	Rock mass quality
R03-RC01	18.5	Limestone	96	80	33	13	poor
R03-RC02	18.5	Mudstone	83	26	16	NI	very poor
	19.5	Limestone	83	26	16	8	very poor
R03-RC03	18.5	Mudstone	100	52	16	14	very poor
R11-CP01	8	Limestone	51	35	27	7/NI	poor
	9.45		100	74	23	14	poor
	11		100	95	95	1	fair
R11-CP03	4.4	Limestone	100	63	50	8	fair
	5		100	85	58	26/6	fair

ROCK STRENGTH

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Bulk density Mg/m ³	UCS MPa	Load failure (KN)	Failure Mode
UCS	R11-CP03	Limestone	C	4.48	0.30	2.71	49.50	154.40	brittle
	R11-CP01A	Limestone	C	11.00	3.20	2.65	31.30	100.60	brittle
	Glasnevin BH01	Limestone	-	20.90	1.80	2.72	66.20	500.30	axial splitting
	Glasnevin BH01	Limestone	-	28.25	1.10	2.70	79.10	608.60	axial splitting
	Glasnevin BH01	Limestone	-	29.60	0.80	2.65	82.50	653.40	axial splitting
	Glasnevin BH01	Limestone	-	30.70	2.70	2.73	22.50	172.30	axial splitting
	Glasnevin BH02A	Limestone	-	32.10	1.70	2.70	79.90	640.40	axial splitting
	Glasnevin BH02A	Limestone	-	33.10	2.40	2.70	92.40	743.00	axial splitting
	Glasnevin BH02A	Limestone	-	33.90	2.10	2.68	66.40	530.10	axial splitting
	Metrolink BH01	Limestone	C	19.75	0.10	-	-	-	-
	Metrolink BH01	Limestone	-	22.50	1.10	2.64	39.70	320.80	-
	Metrolink BH01	Limestone	C	36.30	0.50	2.68	26.70	215.80	-
	Metrolink BH02	Limestone	C	23.00	0.70	2.73	39.10	315.40	-
	Metrolink BH02	Limestone	C	33.40	0.20	2.70	43.20	348.30	-

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Bulk density Mg/m ³	PLT	Point Load index	Load failure (KN)	Failure Mode	Conversion factor	UCS MPa
Point Load Test	Glasnevin BH01	Limestone	-	21.20	0.60	-	1.99	2.72	19.89	-	14.70	29.25
	Glasnevin BH01	Limestone	-	26.40	0.30	-	3.00	4.03	27.69	-	14.70	44.10
	Glasnevin BH01	Limestone	-	28.10	0.60	-	1.42	1.94	14.22	-	14.70	20.87
	Glasnevin BH01	Limestone	-	28.50	0.50	-	1.43	1.83	10.75	-	14.70	21.02
	Glasnevin BH01	Limestone	-	29.50	0.20	-	2.12	2.90	21.23	-	14.70	31.16
	Glasnevin BH01	Limestone	-	29.90	0.50	-	1.17	1.59	11.66	-	14.70	17.20
	Glasnevin BH01	Limestone	-	30.60	2.70	-	1.14	1.48	9.01	-	14.70	16.76
	Glasnevin BH01	Limestone	-	30.95	3.00	-	0.28	0.38	2.81	-	14.70	4.12
	Glasnevin BH02A	Limestone	-	30.80	2.00	-	1.79	2.42	17.17	-	14.70	26.31
	Glasnevin BH02A	Limestone	-	32.00	3.00	-	0.29	0.39	2.93	-	14.70	4.26
	Glasnevin BH02A	Limestone	-	33.60	2.60	-	1.18	1.34	5.22	-	14.70	17.35
	Glasnevin BH02A	Limestone	-	34.35	3.40	-	0.46	0.51	1.76	-	14.70	6.76
	Glasnevin BH02A	Limestone	-	34.60	1.60	-	2.35	3.25	24.94	-	14.70	34.55

Test	Borehole	Soil	Sample type	Top depth (m)	Moisture Content %	Bulk density Mg/m ³	Max Tensile Strenght MPa	BTS	Load failure (KN)	Failure Mode	Conversion factor	UCS MPa
Brazil test	Glasnevin BH01	Limestone	-	20.75	1.00	2.63	3.17	3.17	95.10	Satisfactory	13.70	43.47
	Glasnevin BH01	Limestone	-	26.20	1.60	2.68	7.44	7.44	116.50	Satisfactory	13.70	101.98
	Glasnevin BH02A	Limestone	-	32.90	2.10	2.62	5.15	5.15	83.70	Satisfactory	13.70	70.53

8.3 Characteristic compressive resistance of piles

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 01
Details	Borehole R16-CP01

FORMULATION

Design compressive resistance of a pile, $R_{c,d}$:

$$R_{c,d} = R_{s,d} + R_{b,d} \geq F_{c,d}$$

where:

$F_{c,d}$: design value of the effects of actions (compression)

$$F_{c,d} = \frac{F_{c,k}}{\gamma_F}$$

γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_m : model factor

$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum_i A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

α : adhesion factor (from 1 or higher for very soft clays to 0.2 for very stiff clays).

c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
N_c	9	-

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.50	m
Piles length	11.00	m
A_s	1.57	m ² /m
A_b	0.20	m ²

Lithology	Thickness	From (m)	To (m)	* c_u (kPa)
Made Ground Gravel	1	0	-1	0
Made Ground Gravelly Clay	1	-1	-2	52
Made Ground Gravelly Clay	1	-2	-3	58.5
Made Ground Gravelly Clay	1	-3	-4	52
Made Ground Gravelly Clay	1	-4	-5	32.5
Made Ground Gravelly Clay	5	-5	-10	325
	0			
	0			
	0			
	0			
Limestone (Bedrock)	10	-10	-20	600

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	294	kN
Variable Load	Q_k	623	kN

EC7 - DA1 C1			
A1+M1+R1			
Design ground properties (M)			
Undrained shear strength	γ_{cu}	1.00	
Design resistances (R)			
Partial factor for base resistance	γ_b	1.00	
Partial factor for shaft resistance	γ_s	1.00	
Model factor	γ_m	1.75	
Design actions (A)			
Permanent load factor (fav)	γ_G	1.00	
Permanent load factor (unfav)	γ_G	1.35	
Variable load factor	γ_Q	1.50	
Partial factor on the effects of action	γ_F	1.00	

EC7 - DA1 C2			
A2+M1+R4			
Design ground properties (M)			
Undrained shear strength	γ_{cu}	1.00	
Design resistances (R)			
Partial factor for base resistance	γ_b	1.30	
Partial factor for shaft resistance	γ_s	1.30	
Model factor	γ_m	1.75	
Design actions (A)			
Permanent load factor (fav)	γ_G	1.00	
Permanent load factor (unfav)	γ_G	1.00	
Variable load factor	γ_Q	1.30	
Partial factor on the effects of action	γ_F	1.00	

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

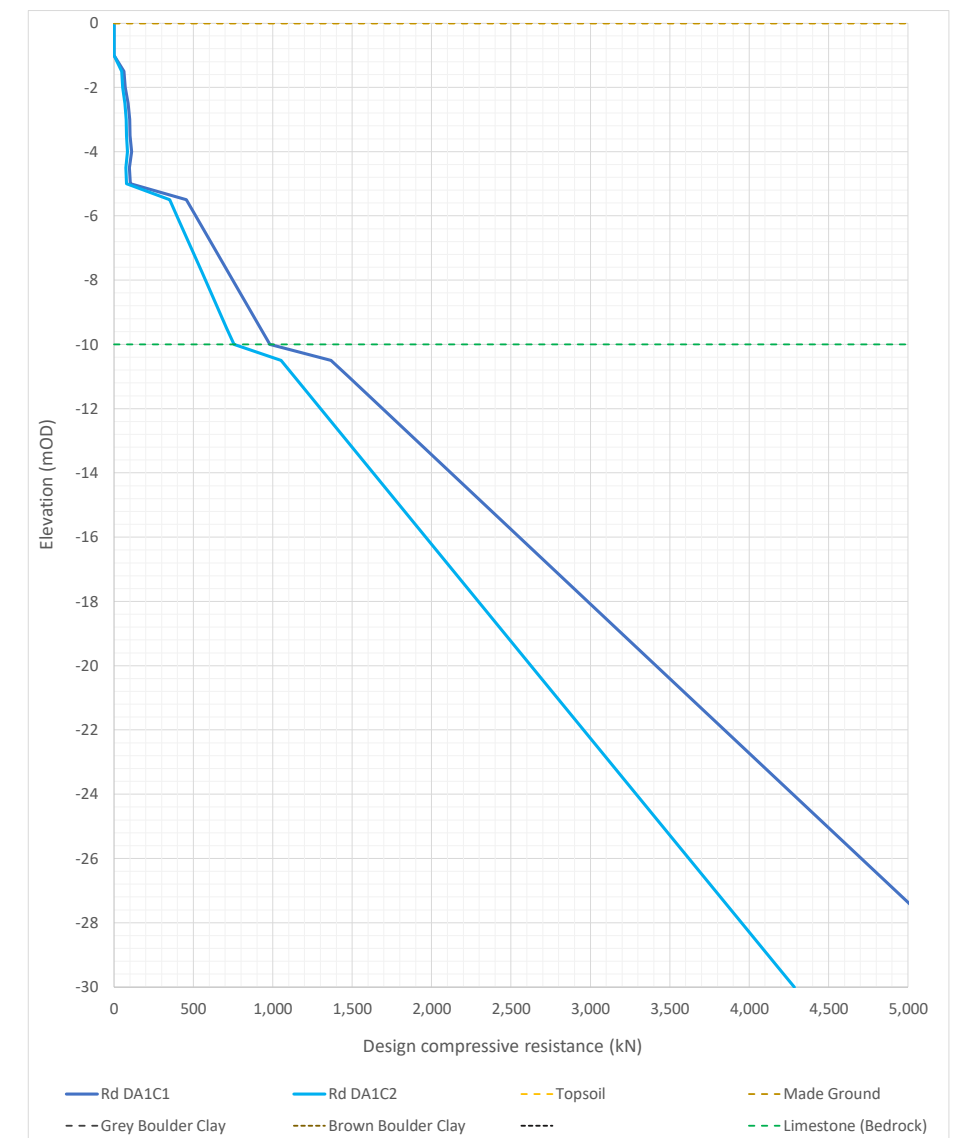
$R_{c,d}$	1475 kN
$F_{c,d}$	1331 kN

$R_{c,d} > E_{c,d}$ OK

$R_{c,d}$	1134 kN
$F_{c,d}$	1104 kN

$R_{c,d} > E_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES



DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 01
Details	Borehole R16-CP02

FORMULATION

Design compressive resistance of a pile, $R_{c,d}$:

$$R_{c,d} = R_{s,d} + R_{b,d} \geq F_{c,d}$$

where:

$F_{c,d}$: design value of the effects of actions (compression)

$$F_{c,d} = \frac{F_{c,k}}{\gamma_F}$$

γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_m : model factor

$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

α : adhesion factor (from 1 or higher for very soft clays to 0.2 for very stiff clays).

c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
N_c	9	-

Lithology	Thickness	From (m)	To (m)	* c_u (kPa)
Topsoil	0.5	0	-0.5	0
Made Ground Gravelly Clay	1.5	-0.5	-2	0
Made Ground Gravelly Clay	1	-2	-3	39
Void	2.3	-3	-5.3	0
Made Ground Gravel	0.7	-5.3	-6	110.5
Gravel	4	-6	-10	325
	0			
	0			
	0			
	0			
Limestone (Bedrock)	10	-10	-20	600

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.50	m
Piles length	11.50	m
A_s	1.57	m ² /m
A_b	0.20	m ²

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	294	kN
Variable Load	Q_k	623	kN

EC7 - DA1 C1		
A1+M1+R1		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.00
Partial factor for shaft resistance	γ_s	1.00
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.35
Variable load factor	γ_Q	1.50
Partial factor on the effects of action	γ_F	1.00

EC7 - DA1 C2		
A2+M1+R4		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.30
Partial factor for shaft resistance	γ_s	1.30
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.00
Variable load factor	γ_Q	1.30
Partial factor on the effects of action	γ_F	1.00

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

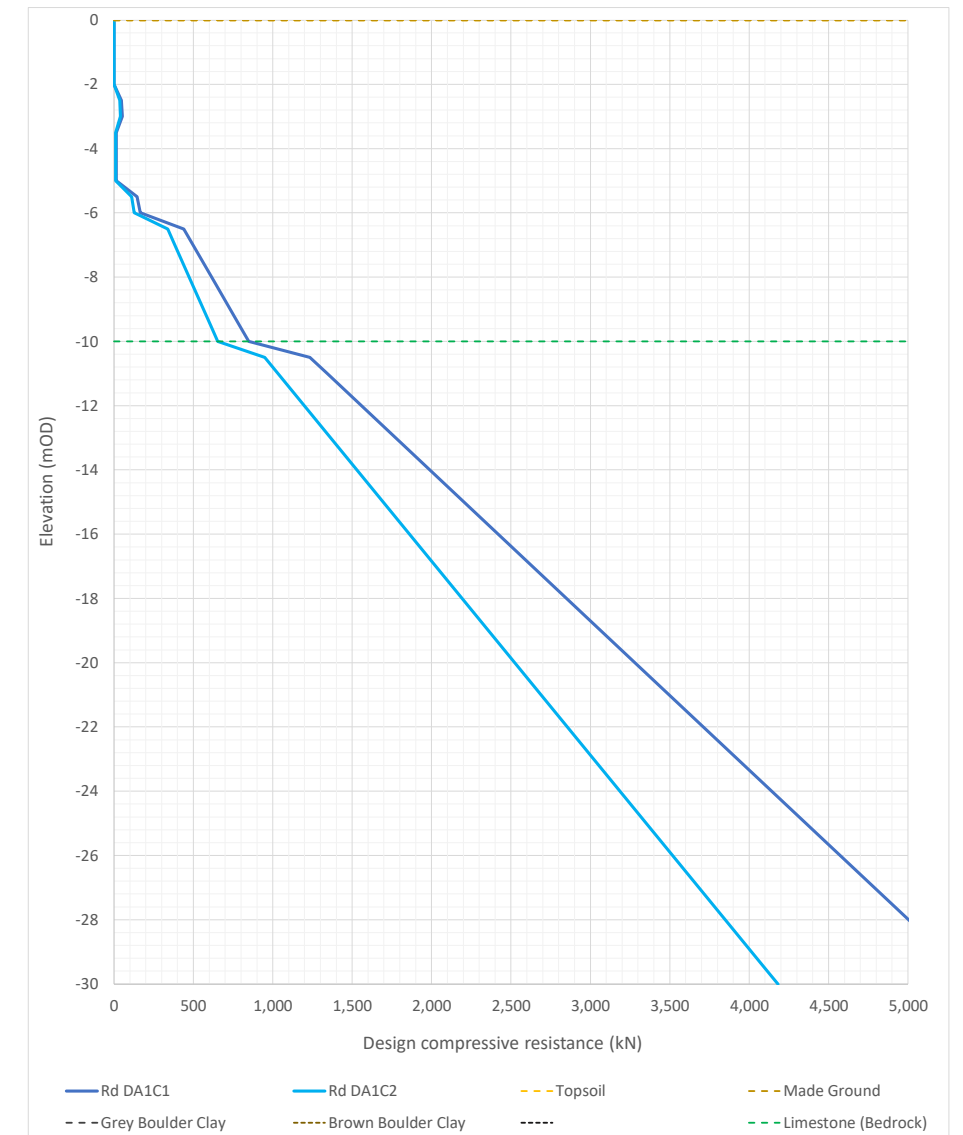
$R_{c,d}$	1449 kN
$F_{c,d}$	1331 kN

$R_{c,d} > F_{c,d}$ OK

$R_{c,d}$	1115 kN
$F_{c,d}$	1104 kN

$R_{c,d} > F_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES



DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 02
Details	Borehole R16-CP03

FORMULATION

Design compressive resistance of a pile, $R_{c,d}$:

$$R_{c,d} = R_{s,d} + R_{b,d} \geq F_{c,d}$$

where:

$F_{c,d}$: design value of the effects of actions (compression)

$$F_{c,d} = \frac{F_{c,k}}{\gamma_F}$$

γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_m : model factor

$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

α : adhesion factor (from 1 or higher for very soft clays to 0.2 for very stiff clays).

c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
N_c	9	-

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.20	m
Piles length	11.50	m
A_s	0.63	m ² /m
A_b	0.03	m ²

Lithology	Thickness	From (m)	To (m)	*Cu(kPa)
Topsoil / Made Ground	1	0	-1	0
Made Ground Sand	1	-1	-2	39
Made Ground Sand	1	-2	-3	32.5
Gravelly Clay	1	-3	-4	71.5
Gravel	1	-4	-5	32.5
Gravel	2	-5	-7	52
Soft Grey Clay	2	-7	-9	32.5
Soft Grey Clay	2	-9	-11	19.5
Gravel	4	-11	-15	325
	0			
Limestone (Bedrock)	5	-15	-20	600

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	0	kN
Variable Load	Q_k	50	kN

EC7 - DA1 C1		
A1+M1+R1		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.00
Partial factor for shaft resistance	γ_s	1.00
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.35
Variable load factor	γ_Q	1.50
Partial factor on the effects of action	γ_F	1.00

EC7 - DA1 C2		
A2+M1+R4		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.30
Partial factor for shaft resistance	γ_s	1.30
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.00
Variable load factor	γ_Q	1.30
Partial factor on the effects of action	γ_F	1.00

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

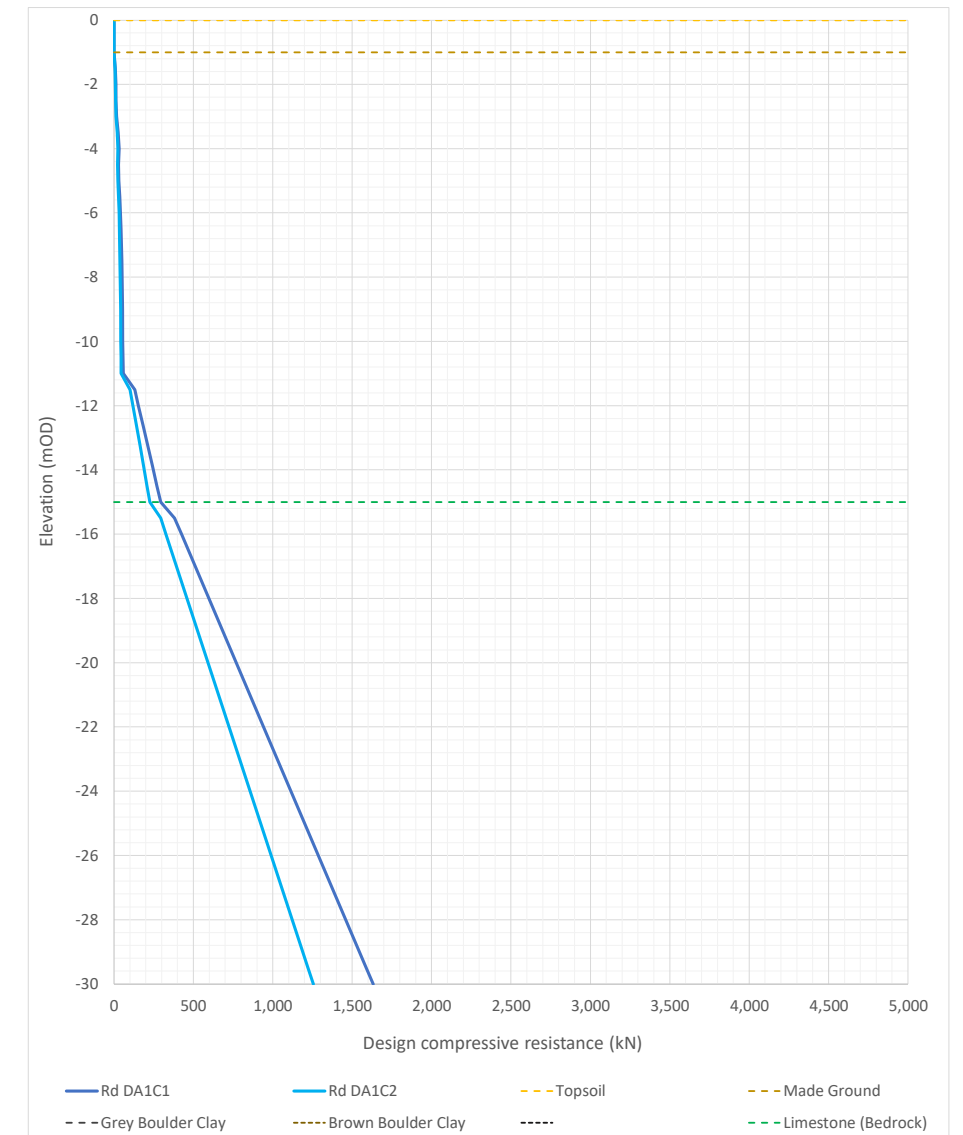
$R_{c,d}$	131 kN
$F_{c,d}$	75 kN

$R_{c,d} > E_{c,d}$ OK

$R_{c,d}$	101 kN
$F_{c,d}$	65 kN

$R_{c,d} > E_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES



DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 02
Details	Borehole R16-CP03

FORMULATION

Design compressive resistance of a pile, $R_{c,d}$:

$$R_{c,d} = R_{s,d} + R_{b,d} \geq F_{c,d}$$

where:

$F_{c,d}$: design value of the effects of actions (compression)

$$F_{c,d} = \frac{F_{c,k}}{\gamma_F}$$

γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_m : model factor

$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

α : adhesion factor (from 1 or higher for very soft clays to 0.2 for very stiff clays).

c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
Nc	9	-

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.20	m
Piles length	12.50	m
As	0.63	m ² /m
Ab	0.03	m ²

Lithology	Thickness	From (m)	To (m)	*Cu(kPa)
Topsoil / Made Ground	2	0	-2	0
Made Ground Gravelly Clay	1	-2	-3	71.5
Made Ground Sand	1	-3	-4	32.5
Sand	1	-4	-5	19.5
Sand	2	-5	-7	13
Soft Grey Clay	2	-7	-9	13
Soft Grey Clay	1	-9	-10	26
Soft Grey Clay	1	-10	-11	13
Soft Grey Clay	1	-11	-12	19.5
Gravel	3	-12	-15	325
Limestone (Bedrock)	5	-15	-20	600

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	0	kN
Variable Load	Q_k	50	kN

EC7 - DA1 C1		
A1+M1+R1		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.00
Partial factor for shaft resistance	γ_s	1.00
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.35
Variable load factor	γ_Q	1.50
Partial factor on the effects of action	γ_F	1.00

EC7 - DA1 C2		
A2+M1+R4		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.30
Partial factor for shaft resistance	γ_s	1.30
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.00
Variable load factor	γ_Q	1.30
Partial factor on the effects of action	γ_F	1.00

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

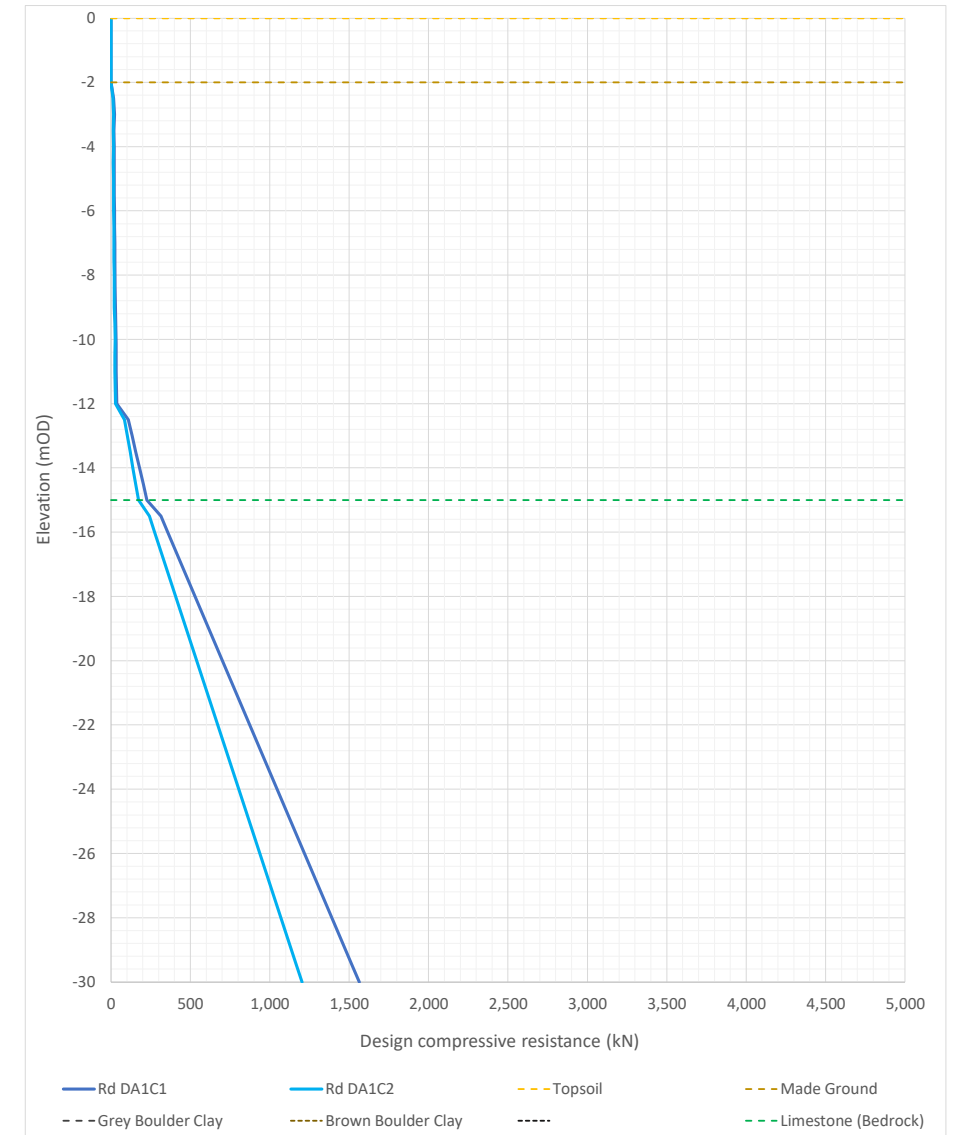
$R_{c,d}$	109 kN
$F_{c,d}$	75 kN

$R_{c,d} > E_{c,d}$ OK

$R_{c,d}$	84 kN
$F_{c,d}$	65 kN

$R_{c,d} > E_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES



DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 03
Details	Borehole R16-CP03

FORMULATION

Design compressive resistance of a pile, $R_{c,d}$:

$$R_{c,d} = R_{s,d} + R_{b,d} \geq F_{c,d}$$

where:

$F_{c,d}$: design value of the effects of actions (compression)

$$F_{c,d} = \frac{F_{c,k}}{\gamma_F}$$

γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_m : model factor

$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

α : adhesion factor (from 1 or higher for very soft clays to 0.2 for very stiff clays).

c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
Nc	9	-

Lithology	Thickness	From (m)	To (m)	*Cu(kPa)
Topsoil / Made Ground	1	0	-1	0
Made Ground Sand	1	-1	-2	39
Made Ground Sand	1	-2	-3	32.5
Gravelly Clay	1	-3	-4	71.5
Gravel	1	-4	-5	32.5
Gravel	2	-5	-7	52
Soft Grey Clay	2	-7	-9	32.5
Soft Grey Clay	2	-9	-11	19.5
Gravel	4	-11	-15	325
	0			
Limestone (Bedrock)	5	-15	-20	600

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.50	m
Piles length	15.50	m
As	1.57	m ² /m
Ab	0.20	m ²

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	210	kN
Variable Load	Q_k	604	kN

EC7 - DA1 C1		
A1+M1+R1		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.00
Partial factor for shaft resistance	γ_s	1.00
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.35
Variable load factor	γ_Q	1.50
Partial factor on the effects of action	γ_F	1.00

EC7 - DA1 C2		
A2+M1+R4		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.30
Partial factor for shaft resistance	γ_s	1.30
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.00
Variable load factor	γ_Q	1.30
Partial factor on the effects of action	γ_F	1.00

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

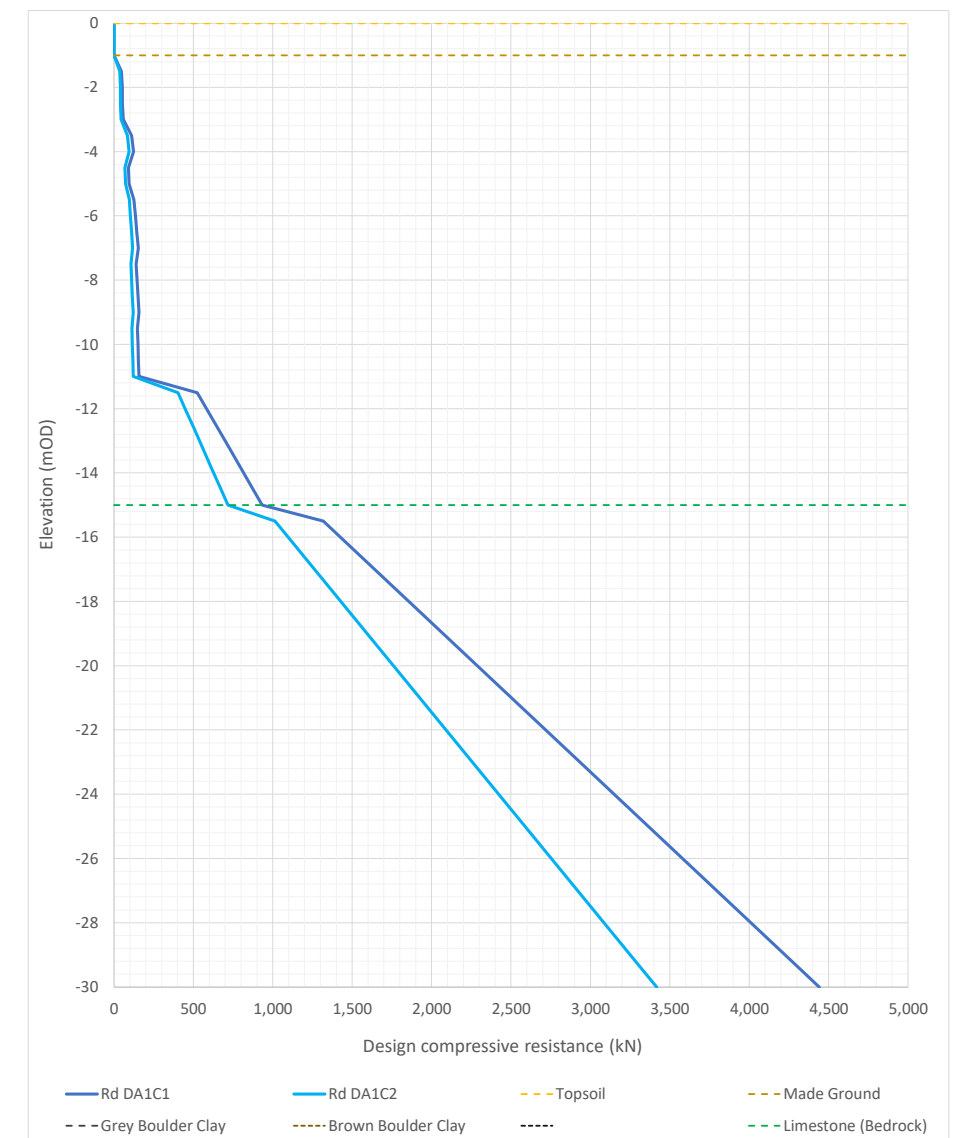
$R_{c,d}$	1318 kN
$F_{c,d}$	1190 kN

$R_{c,d} > F_{c,d}$ OK

$R_{c,d}$	1014 kN
$F_{c,d}$	995 kN

$R_{c,d} > F_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES



DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

According to Eurocode 7 by calculation from ground parameters and Irish National Annex

(Valid for piles spaced at 3 diameters center to center or greater)

Project	RD5862 Dublin BusConnect
Structure	Ringsend 03
Details	Borehole R16-CP03

FORMULATION

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γ_F : partial factor on actions or effects of actions

$R_{s,d}$: Design value of shaft resistance

$$R_{s,d} = \frac{R_{s,k}}{\gamma_s \cdot \gamma_m}$$

$R_{b,d}$: design value of base resistance

$$R_{b,d} = \frac{R_{b,k}}{\gamma_b \cdot \gamma_m}$$

γ_s : partial factor for shaft resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

γ_b : partial factor for base resistance derived from National Annex. It depends on the type of piles (driven, bored or CFA).

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$R_{s,k}$: characteristic shaft resistance

$$R_{s,k} = \sum A_{s,i} \cdot q_{s,i,k} = \alpha \cdot c_u \cdot A_{s,i,k}$$

$R_{b,k}$: characteristic base resistance

$$R_{b,k} = A_b \cdot q_{b,k} = N_c \cdot c_u \cdot A_b$$

where:

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c_u : Undrained shear strength

$A_{s,i,k}$: area of the pile shaft (for the stratum under consideration)

N_c : bearing capacity factor ($N_c=9$ provided that the pile has been driven at least to a depth of 5 diameters into the bearing stratum)

A_b,k : area of the pile base

INPUT DATA

SOIL

Ground Level	0	mOD
α	0.4	-
Nc	9	-

FOUNDATION

Foundation level	0	mOD
ϕ_{pile}	0.50	m
Piles length	16.50	m
As	1.57	m ² /m
Ab	0.20	m ²

Lithology	Thickness	From (m)	To (m)	*Cu(kPa)
Topsoil / Made Ground	2	0	-2	0
Made Ground Gravelly Clay	1	-2	-3	71.5
Made Ground Sand	1	-3	-4	32.5
Sand	1	-4	-5	19.5
Sand	2	-5	-7	13
Soft Grey Clay	2	-7	-9	13
Soft Grey Clay	1	-9	-10	26
Soft Grey Clay	1	-10	-11	13
Soft Grey Clay	1	-11	-12	19.5
Gravel	3	-12	-15	325
Limestone (Bedrock)	5	-15	-20	600

Actions			
Favourable Permanent Load	$G_{k, fav}$	0	kN
Unfavourable Permanent Load	$G_{k, unfav}$	210	kN
Variable Load	Q_k	604	kN

EC7 - DA1 C1		
A1+M1+R1		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.00
Partial factor for shaft resistance	γ_s	1.00
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.35
Variable load factor	γ_Q	1.50
Partial factor on the effects of action	γ_F	1.00

EC7 - DA1 C2		
A2+M1+R4		
Design ground properties (M)		
Undrained shear strength	γ_{Cu}	1.00
Design resistances (R)		
Partial factor for base resistance	γ_b	1.30
Partial factor for shaft resistance	γ_s	1.30
Model factor	γ_m	1.75
Design actions (A)		
Permanent load factor (fav)	γ_G	1.00
Permanent load factor (unfav)	γ_G	1.00
Variable load factor	γ_Q	1.30
Partial factor on the effects of action	γ_F	1.00

NOTE: Set M2 is only used to calculate unfavourable design actions on piles e.g. to negative skin friction.

$R_{c,d}$	1363 kN
$F_{c,d}$	1190 kN

$R_{c,d} > F_{c,d}$ OK

$R_{c,d}$	1049 kN
$F_{c,d}$	995 kN

$R_{c,d} > F_{c,d}$ OK

DESIGN COMPRESSION RESISTANCE OF PILES. TOTAL STRESSES

